

## Probability and consequence of post-fire erosion for treatability of water in an unfiltered supply system

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### Key Points:

- Consequence of wildfire for an unfiltered water supply system is modeled based on erosion, stochastic rainfall and reservoir hydrodynamics
- There is a 10-30% chance that water is untreatable for >1 year due to high suspended sediment concentration after a high severity wildfire
- Assumptions about spatiotemporal rainfall patterns, reservoir hydrodynamics, and erosion are all important sources of error in risk

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## **Abstract**

Forested catchments are critical to water supply in major cities. Many of these catchments face the threat of post-wildfire erosion, which can contaminate reservoir water. The aim of this paper is to determine the probability and duration of disruptions to treatability due to runoff-generated debris flows in the first year after a wildfire, before substantial vegetation recovery takes place. We combine models of reservoir hydrodynamics, post-fire erosion, and a stochastic rainfall to determine probability and magnitude of sediment concentration at the reservoir water offtake. Central to the paper is our technique for linking model components into a risk framework that gives probabilities to the number of days that the turbidity threshold for treatment is exceeded. The model is applied to the Upper Yarra reservoir, which is the linchpin the water supply system for Melbourne in SE Australia. However, the framework is applicable to other unfiltered water supply systems where suspended sediment are a risk to treatability. Results show that post-wildfire erosion poses a substantial threat, with a relatively high probability (annual exceedance probability = 0.1- 0.3) of water being untreatable for > 1 year following a high-severity wildfire. Important factors that influence the risk include post-wildfire runoff potential, reservoir temperature and the amount of clay-sized grains in eroding headwaters. Assumptions about spatial-temporal rainfall attributes, reservoir hydrodynamics, and the catchment erosion potential are all important sources of error in our estimate of risk. Our approach to risk quantification will help support planning, risk management and strategic investment to mitigate impacts.

**Keywords:** wildfire, treatability, water supply, debris flow, reservoir hydrodynamics

## 1. Introduction

Large cities around the world source their drinking water from forested catchments, which deliver water that is treatable at minimal cost. These forests are often prone to wildfire, which tend to increase surface runoff, destabilize soils and trigger increased sediment delivery into water reservoirs [Emelko *et al.*, 2011; Hohner *et al.*, 2019; Sheila F. Murphy *et al.*, 2015; Rhoades *et al.*, 2011; Smith *et al.*, 2011]. The consequences of wildfire for water supply are a significant concern, with the potential for extended periods of water supply disruption, increased water treatment cost. There is a demand for models to inform catchment restoration, fuel management, and investment in treatment infrastructure [Bladon *et al.*, 2014; Hohner *et al.*, 2016; Jones *et al.*, 2017; Smith *et al.*, 2011; White *et al.*, 2006; Writer *et al.*, 2014].

With increased data availability and improved models, the impacts of wildfire on erosion and sediment delivery from hillslopes and catchments can be predicted with some confidence, or at the least with a specified measure of uncertainty [Langhans *et al.*, 2016; McGuire *et al.*, 2016; Robichaud *et al.*, 2016; Vieira *et al.*, 2014]. Some of these models can be used to assess threats to water resources, currently and into the future with different management strategies [e.g. Gould *et al.*, 2016; Jones *et al.*, 2017; Sankey *et al.*, 2017]. While, generally, there are sophisticated modelling frameworks linking climate, hydrology and water supply management that account for turbidity [e.g. Porter *et al.*, 2015], there are few attempts at translating models of post-fire catchment processes to impacts on treatability of reservoir water. Post-fire erosion models are therefore underutilized when it comes to informing reservoir managers about risk.

Metrics of risk provide a quantitative means for cost-effective and evidence-based mitigation [Khan *et al.*, 2015; Nyman *et al.*, 2013; Thompson and Calkin, 2011]. In the context of wildfire and treatability of source water, we here define risk as the probability, severity and duration of contamination events at the reservoir offtake, where water is extracted for treatment. With this definition of risk, the consequence of post-wildfire erosion for water supply is a function of i) frequency and magnitude of erosion in headwaters, ii) the transport processes of sediment to the reservoir, and iii) ultimately the propagation of pollutant plume from input location to water offtake. Developing a predictive model of these processes is a large undertaking in terms of data needed for parametrizing and testing models of catchment erosion and reservoir hydrodynamics. However, connecting erosion processes in catchments with sediment transport in reservoirs is paramount to planning and preparedness, and it is needed to provide water supply managers with suitable metrics to evaluate cost and benefits of different options for mitigation and adaptation [Khan *et al.*, 2017]. Options include investment in fuel reduction, active fire-management (suppression), filtration capacity, diversifying water sourcing options, and prevention of particulates reaching the reservoir [Khan *et al.*, 2015; Thompson and Calkin, 2011].

In this study, we are motivated by the need amongst water supply managers to know '*For how long is it likely that my reservoir will be offline due to contamination by post-wildfire erosion?*' The aims of this research are therefore to:

1. Develop a model to quantify the probability and duration of disruption to treatability due to debris flows in the first year after wildfire, before substantial vegetation recovery takes place.
2. Apply the model to the Upper Yarra catchment, which forms the linchpin of the water supply system for the city of Melbourne in SE Australia.

3. Evaluate the model results in terms of risk to Melbourne's water supply, the sources of error in model estimates, and the implications of our results for mitigation.

Central to our modelling approach is the coupling of reservoir hydrodynamics, post-fire erosion potential, and stochastic rainfall to estimate duration and probability that turbidity thresholds for treatability are exceeded at the reservoir offtake. Note that in this study we only consider sediment in our risk model. Unlike most water supplies for developed countries, the majority of the water supply for the city of Melbourne is from forested catchments, and while it is disinfected, it is unfiltered, therefore relatively small changes in color and turbidity may have a significant impact on drinking water quality. These impacts can be on aesthetics, but more importantly, can impact disinfection efficacy. Impacts on water supply due to elevated dissolved organic carbon, reduced flocculation, and toxic disinfection byproduct [e.g. *Hohner et al.*, 2019; *Stone et al.*, 2011; *Writer et al.*, 2014] are not considered in this study. However, the framework presented is useful in developing risk metrics that comprise these other sources of contamination, which are linked to hillslope processes and reservoir hydrodynamics [*Murphy et al.*, 2015; *Writer et al.*, 2014].

## **2. Model framework**

The model is an implementation of the conceptual framework by *Nunes et al.*, [2018] for assessing water contamination risk. It includes three major components for predicting the probability and duration of treatability disruptions:

1. Propagation of fine sediment within the reservoir, from entry points to the offtake.
2. Susceptibility of the landscape to extreme erosion events.
3. Stochastic spatial-temporal rain fields which cause erosion.



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been linked to nearly all major water quality concerns in the Victorian uplands (SE Australia) following several large wildfire that have occurred in 2003, 2007, 2015 and 2009 [Lyon and O'Connor, 2008; Nyman *et al.*, 2011; Smith *et al.*, 2011; White *et al.*, 2006]. Once delivered to the stream network, the fine-grained material produced by debris flows is efficiently transported in suspension to downstream assets [Lyon and O'Connor, 2008; Nyman *et al.*, 2020]. In their sediment budget for a post-fire debris flow in a geomorphic setting (steep dissected uplands with confined valleys) similar to the Melbourne's water supply catchments, Nyman *et al.* [2020] found that for silt and clay-sized grains, only ~10% of eroded sediments were deposited in the drainage networks linking debris-flow producing headwaters and streams. The justification for developing the model around debris flow as the dominant process for assessing water quality risk in Melbourne's water supply catchment is described in Langhans *et al.* [2016], where the catchment response model was developed.

### 3 Methods

#### 3.1. Study area

The model was applied to the Upper Yarra (UY) catchment (~337 km<sup>2</sup>), comprising mainly sedimentary rocks, and located in the southern region of the Eastern Uplands geomorphic region of Victoria in SE Australia (Figure 2). The climate is temperate with warm, dry summers and cool, wet winters. With strong rainfall seasonality and eucalyptus-dominated vegetation, the region supports some of the most productive and flammable forest ecosystems on earth [Bowman *et al.*, 2009]. The UY catchment is completely covered by forests.

However, with a relief of 870 m (elevation range: 321-1191 m.a.s.l.) there are large differences in rainfall and vegetation from low to high elevations. Typically, at lower elevations the vegetation consists of open and dry mixed-species *Eucalyptus* forests. At higher elevations, or on polar-facing (mesic) slopes, the catchment consists of tall and damp



notice, and large costs of sourcing alternative supplies (e.g. desalination water). The hydrodynamics of the UY reservoir are dominated by strong summer thermal stratification and deep winter mixing

### 3.2. Reservoir hydrodynamics model

The propagation of the sediment plume from input location to dam offtake was modelled using the three-dimensional Aquatic Ecosystem Model (AEM3D), which is an updated version of the Estuary and Lake COmputer Model (ELCOM). The model solves the unsteady Reynolds-averaged Navier-Stokes equations and scalar transport equations with the Boussinesq approximation and hydrostatic pressure terms using the numerical methods described by *Casulli and Cheng*, [1992] and *Hodges et al.*, [2000]. Previous applications of ELCOM are widely referenced in limnological studies [*Trolle et al.*, 2012], including studies of reservoir hydrodynamics and sediment transport [*Dissanayake et al.*, 2019; *Trolle et al.*, 2012]. AEM3D features in more recent publications describing reservoir and lake modelling [e.g. *Madani et al.*, 2020; *Ryu et al.*, 2020; *Tranmer et al.*, 2018; *Valipour et al.*, 2019; *Valerio et al.*, 2019; *Zamani and Koch*, 2020]. *Zamani and Koch*, [2020] demonstrate the appropriateness of AEM3D for modelling reservoirs, such as Upper Yarra Reservoir, that have complex bathymetry, steep walls and a wind-mixed surface layer. *Ryu et al.*, [2020] use measurements of water level, flow velocity, temperature and conductivity to demonstrate the ability of the model to predict mixing of density flows in a reservoir.

Sediment concentrations in AEM3D are prescribed in the inflow boundary conditions and calculated in each model cell after accounting for transport, vertical mixing, settling and resuspension. Concentrations are tracked in groups (e.g. fine clay, clay, silt, fine sand, etc) of equal diameter and density and are aggregated to determine total sediment concentrations.

Sediment transport is modelled using the mass-conservative ULTIMATE-QUICKEST scalar transport solver [Leonard, 1991] based on the velocity field derived by the hydrodynamics solver. Vertical mixing is modelled using a 3D mixed-layer energy budget approach that includes terms for convective mixing, wind mixing and internal and boundary shear [Hodges *et al.*, 2000]. Flocculation was not considered in this study, so the diameter and density of the inflowing sediments do not change in the reservoir. It has been assumed that any contribution from resuspension is negligible, which is a reasonable assumption given that Upper Yarra is a deep reservoir with low bottom shear stress. Sediment density and concentration contribute to water density so that high inflow sediment concentrations increase the plunging depth of inflow intrusions.

AEM3D (and previously ELCOM) models have been developed for Melbourne Water's key water supply reservoirs [e.g. Morillo *et al.*, 2006; Marti *et al.*, 2011] as components in an overarching decision support system [Mills *et al.*, 2011]. A model for Upper Yarra Reservoir was first developed in 2006 to assess the dynamics of inflow intrusions and was calibrated and validated using data collected with a combination of thermistor chains and a fine-scale profiler [Dallimore and Imberger, 2006]. Further verification of the model performance was undertaken when the first large post-drought inflow into Upper Yarra Reservoir was observed in June 2012 after a storm in the catchment [Stevens *et al.*, 2012].

### 3.3 Post-fire debris flow model

The model of probability and magnitude of sediment delivery from runoff generated debris flows to the reservoir is based on earlier work by Langhans *et al.* [2016] and Nyman *et al.* [2015]. Model structure, calibration and validation are described in the Supporting Information. In summary, debris flow occurrence is determined from a threshold stream power ( $\Omega_{crit}$ ) at the outlet of small zero-order headwater (~2 ha). The stream power,  $\Omega$ , is a

function of discharge and slope and the threshold,  $\Omega_{crit}$ , varies depending on the mass of non-cohesive surface material ( $m_{nc}$ ) that is available on the hillslopes. If  $m_{nc}$  is high, the  $\Omega$  required to incise the channel and trigger a debris flow is lower than if the mass is low. This concept is based on the assumption that the initiation of runoff-generated debris flows is caused by combined shear stresses of water flow and granular fronts formed by hillslope debris flows [Langhans *et al.*, 2017]. The production of surface runoff is modelled with the kinematic wave equation in KINEROS2 [Woolhiser *et al.*, 1990] and controlled by soil hydraulic parameters that vary as functions of fire severity (change in normalized burn ratio, dNBR) and aridity index (AI), both of which are related to the soil hydraulic properties that control infiltration [Moody *et al.*, 2015; Van der Sant *et al.*, 2018]. The magnitude of erosion by debris flows were estimated based on empirical relations in Nyman *et al.*, [2015], whereby erosion on hillslopes and in channels is a function of a stream power index (product of local slope,  $S$  ( $\text{m m}^{-1}$ ), and contributing area,  $A$  ( $\text{m}^2$ ) (Equations (4-8) and Figure S1 in Supporting Information).

### 3.4 Model implementation in the Upper Yarra Reservoir

#### 3.4.1 Reservoir hydrodynamics: transfer function and model sensitivity

The propagation of sediment plumes in the UY reservoir were simulated in AEM3D using a range of potential debris flow scenarios. Scenarios varied in terms of debris flow sediment delivery location (one of four different locations shown in Figure 2b) and sediment load. The objective was to model the propagation of the resultant sediment plumes and output a timeseries of predicted sediment concentration at the offtake for each scenario. A suitable range of sediment loads was determined by considering 5 levels of erosion response, ranging from the lowest (in the case of a very small area ( $\sim 30$  ha) producing debris flows) to the highest possible (all contributing headwaters producing debris flow). This approach results in

transfer functions that relate sediment loads from the catchments to sediment concentration at the water offtake for a wide range of possible debris flow responses that could eventuate after a wildfire. The range of sediment load scenarios are developed based on assumption about the number of debris flow producing catchments and sediment load estimates from the model described in Figure S1 in Supporting Information). The propagation of sediment plumes was simulated for sediment delivery events at 4 input locations (Figure 2b) that represent a range of possible scenarios of debris flow response in headwaters. Discharge during the sediment delivery events was based on the maximum rainfall depth (35 mm) reported by *Nyman et al.* [2015] for post fire debris flows in Victoria, and by assuming that 100% of runoff from the headwater areas generated debris flows. This will overestimate runoff in the debris flow producing headwaters, however this is offset by the underestimation of runoff from adjacent burned but non-debris flow producing areas that are not currently accounted for in this approach. The combination of debris flow loads, and discharge used in the simulations generate total debris flow sediment concentrations of  $\sim 570 \text{ g L}^{-1}$  and sediment concentrations of clay of  $\sim 20\text{-}40 \text{ g L}^{-1}$ . These are reasonable given the known sediment concentrations in streams below debris flows producing headwaters: 36-59  $\text{g L}^{-1}$  at Porepunkah [*Leak et al.*, 2003] and 112 -143  $\text{g L}^{-1}$  at Snowy Mountains [*Brown*, 1972].

Post-fire sediment loads were divided into 22 classes of grain diameter ranging from 0.001 to 30 mm. The density of the sediment particles was set at  $2650 \text{ kg m}^{-3}$  for fines (1.5 mm or less) and  $2220 \text{ kg m}^{-3}$  for coarser grains ( $> 1.5 \text{ mm}$ ). Once entering the reservoir in the AEM3D model at the specific boundary location, the concentration of each of the 22 classes was calculated separately in each model grid cell, as a function of the transport into and out of the cell by background flow and settling rate (determined from particle size, density and following Stokes Law settling velocity calculations). Buoyancy effects associated with plume

density, determined as a function of inflow temperature and sediment load, are considered by the model and therefore modify the computed flow field.

The grain size distribution of the sediment delivered to the reservoir by erosion events (Figure S2) was calculated based on data collected during surveys of 10 debris flows in eastern uplands of Victoria [Nyman *et al.*, 2015] (locations are shown in Figure 2), at sites with similar lithology to that of the UY catchment. For each sediment input scenario, AEM3D was run with meteorological forcing provided by observations at 2m above the lake surface taken at 9 min intervals using a moored Lake Diagnostic System (LDS) installed in UY reservoir in September 2005. Data measured in 2006 were selected as representing typical summer conditions over a period of reliable and complete measurements. Inflow and outflow rates monitored by the Melbourne Water SCADA system were used for the reservoir water balance. Thermistor chain data from the LDS and boat deployed CTD profiles have been used in previous studies to calibrate the model [Dallimore and Imerger, 2006].

For each sediment input scenario, the number of days that sediment concentration exceeded treatability thresholds at the reservoir offtake was recorded, indicating the period of likely disruptions to treatability. Historically, turbidity at the reservoir offtake is usually under 2 NTU and while excursions above this in the short term are tolerable, turbidities over 5 NTU [likely between 5 and 10 mg L<sup>-1</sup>: Lane *et al.*, 2006] are regarded as unacceptable from both a disinfection and an aesthetic perspective and not recommended by the Australian Drinking Water Guidelines (ADWG 2011). For our modeling, we therefore set the threshold for treatment at the range of 5-10 mg L<sup>-1</sup>. The duration (in days) that sediment concentration exceeded this threshold range was plotted against the input sediment load for all five scenarios at each input location. An empirical function was fitted to these outputs and used as

transfer function that relates erosion responses to days of undeliverable water. The sensitivity of sediment propagation to parameter inputs was examined in relation to the treatability threshold, grain size distribution, seasonality, and temperature.

#### 3.4.2 Debris flow susceptibility for two wildfire scenarios

The debris flow response model provides a look-up table containing critical stream power ( $\Omega_{crit}$ ) values for combinations of dNBR, slope and aridity of zero-order headwater for the first year after the fire. Loads,  $M_{df}$  [t], for each location of a potential debris flow, were estimated at the outlet of first-order channels based on erosion in contributing channels and hillslopes. It is assumed that deposition and erosion in the higher order stream network has little impact on the overall clay loads being delivered to the reservoir. This assumption is supported by findings in *Nyman et al.*, [2020], where measurements and modelling show that a large fraction of clay is generated from hillslopes and colluvium in first-order headwaters. However, expanding on the erosion models to provide a more dynamic link between headwaters and reservoir is an important area for further model development. Values of  $M_{df}$  were pre-processed and kept in the lookup table alongside variables related to  $\Omega_{crit}$  for each zero-order headwater. The approach of using a lookup table and pre-processed loads to represent potential debris-flow response allows for efficient calculation of magnitude and frequency using Monte Carlo simulations, that consider random variation in fire severity and rainfall, as described below.

Scenarios of post-fire debris flow were simulated using fire severity distributions from the Kilmore-Murrindindi (KM) fire that burned in catchments adjacent to the UY catchment during the Black Saturday wildfires in February 2009 (Figure 2a). The wildfire provides fire severity distributions for two distinct periods of burning, representing contrasting fire

behavior in forests that are typical of the UY catchment. The two distinct periods of burning are associated with different fire weather. During the first 12 hours of burning, the fire weather was extreme; high wind speeds from the northwest, followed by an abrupt weather change with strong and gusty winds from the southwest [Cruz *et al.*, 2012]. Extreme fire behavior, with crown scorch and burning, persisted until about 5 hours after the weather change. After this time, the fire continued to burn for several weeks during cooler and less windy conditions, resulting in comparably low burn severities, mostly in the understory vegetation (Figure 2a). For our simulations, we consider the first and second periods of burning in the KM fire to represent contrasting fire types: one dominated by crown fire and the other dominated by understory fire.

Fire severity distributions for the low- and high- severity (understory vs crown fire) scenarios were produced using the difference normalized burn ratio (dNBR; calculated from Landsat) as a severity metric (Figure 3). The mean dNBR was calculated for individual first-order headwaters that correspond with model response units in Hydrofire. For the high and low severity periods of burning, the number of samples in the KM fire were similar (5414 and 4662 headwaters, respectively). The dNBR were analyzed to determine if there were trends with forest type, aridity, elevation and aspect. No trends were detected. Scenarios of low and high burn severities in the UY catchment were therefore simulated by randomly allocating values to headwaters based on the two distributions, corresponding to low and high severity fires. All headwaters in the catchment were assigned a fire severity, assuming in the simulations that the wildfire affects the entire catchment area. Maps of critical 12-minute rainfall intensities ( $I_{12\_crit}$ ) in individual headwaters were produced taking the median dNBR from 10 random samples from the distributions in Figure 3. Maps of the median annual exceedance probability (AEP) was determined from the median  $I_{12\_crit}$  and design storm



from 0.1 to 0.3 [Nyman *et al.*, 2015], and the area of high erosion potential in the UY catchment is small relative to the size of storm cells. The debris flow conditions, which we assume to be the main source of risk, are therefore unlikely to occur multiple times in the same location during the recovery period.

When simulating erosion with design storms, the rainfall events were sampled at a random location each year using the data grid (~ 2.5 x 2.5 km) with design storm statistics from the Bureau of Meteorology (BoM). Each year, a storm was sampled in each grid cell based on the distribution of annual exceedance probabilities (AEPs) at that cell. Once initiated in a grid cell, the rainfall intensities in surrounding cells decline according to an area reduction factor ( $\lambda$ ), which was calculated from the spatial distribution of  $I_{12}$  in storms events in the radar archive (Figure S6). Design storms were sampled 150 times in each cell. Sampling of storms was repeated 10 times, resampling fire severity on each iteration. The distribution of sediment delivery events for high and low fire severity scenarios was determined in each cell of the AEP grid by plotting the annual loads as a cumulative distribution function (*cdf*) of annual  $M_c$ . The distribution of annual exceedance probability (AEP) for  $M_c$  was then estimated from  $1-cdf$ .

With the rainfall generator, the rainstorms were simulated over the study area at 6-minute timesteps and aggregated into 24-hour peak  $I_{12}$ . A total of 6000 storms ( $I_{12}$  between 20 and 200 mm h<sup>-1</sup>) were created over the catchment. The cell with the highest  $I_{12}$  was used to determine the annual exceedance probability (AEP) of the storm based on design storm parameters, which are available from BoM. The debris flow response in headwaters was determined from the peak daily  $I_{12}$  across the catchment. Thus, each simulated storm has a known AEP and a simulated clay load ( $M_c$ ). With 6000 simulations, we obtain a distribution









consequence for treatability of water at the reservoir offtake. The framework presented in Figure 1 links two models with a transfer function (Figure 5). One model predicts the erosion response of headwaters and the other predicts how sediment is propagated through the reservoir. The erosion response is simulated using two methods for generating stochastic rainfall. The framework for linking probability of erosion and consequence for water supply is a significant conceptual development in terms of extracting value from erosion models applied to the water resources sector. It is a template approach for future studies that aim to derive metrics from post-fire erosion models that can be used to guide cost-benefit analysis, investment and policy in the management of catchments for potable water supply [*Khan et al.*, 2015]. Moreover, our approach is strongly aligned with the framework advocated by *Nunes et al.* [2018] whereby the model development is guided by 1) identifying contaminant and assets of concern, 2) understanding the dominant processes that govern contaminant mobilization, and 3) mapping the pathways linking contaminants to areas of concern.

The approach to modeling water quality risk involves two major decisions regarding the conceptual representation of risk. First, the models of sediment delivery from the catchment and the propagation of sediment within the reservoir are coupled via empirical transfer functions (Figure 5). Creating transfer functions from an array of hydrodynamic modelling results that capture the expected range of post-fire sediment load and input locations provides a means to incorporate stochasticity in fire and erosion without computational constraints imposed by the hydrodynamic model, which is time consuming to run. For instance, a single iteration of AEM3D takes ~ 22 hours running 15 cores on a 3.10 GHz Xeon Linux computer. Moreover, by decoupling the two models, our approach allows for greater model tractability in terms of sources of error and the major levers on risk.

Second, the erosion model only considers runoff-generated debris flows as a cause of sediment delivery to the reservoir. In modelling erosion as a threshold process, we seek to achieve parsimony with respect to the dominant processes driving water quality risk [advocated by *Nunes et al.*, 2018], thereby eliminating unnecessary complexity and providing stronger alignment between key hydrological drivers and threat [*Nyman et al.*, 2013]. The decision to consider runoff-generated debris-flows only was justified in *Langhans et al.* [2016], and based on evidence from past observation of water quality impacts from forested catchments in the region [*Smith et al.*, 2011]. However, we emphasize that the framework is generally applicable to water supply systems and can be tailored to other hydrogeomorphic settings where erosion processes may be governed by different processes or where the constituents of concern are different. The reservoir model (AEM3D) is suited for modelling range constituents and biogeochemical processes [*Tranmer et al.*, 2018], so impacts of post-fire hydrology on water quality in lakes and reservoir can be evaluated with AEM3D more generally in the context of impacts on both ecosystem and water supply. The reservoir hydrodynamics model can be refined and tested further to include other issues that are important for treatability of water after wildfire: water quality constituents other than suspended sediment, flocculation processes, and longer-term issues such as algae bloom that stem from post-wildfire nutrient enrichments [*Emelko et al.*, 2016; *Hohner et al.*, 2019].

### *5.2 Magnitude of risk to treatability and sources of error*

The study demonstrates that after high severity wildfire there is a relatively high probability ( $0.1 < \text{AEP} < 0.3$ ) of major consequences in the UY reservoir whereby water is untreatable for several months to a year. Water quality impacts of such magnitudes have been previously recorded in the SE Australia region after large erosion events [*Ollerenshaw et al.*, 2014; *White et al.*, 2006]. In Lake Bellfield, landslides and debris flows caused 1-2 order of

magnitude increases in turbidity for over one year (2 NTU prior to the erosion event, 38-235 NTU in the first year after the debris flows), and resulted in water that was unsuitable for drinking for at least 3 years [Ollerenshaw *et al.*, 2014]. In Canberra, post-fire erosion including runoff-generated debris flows, caused increased turbidity that made the water unfit for distribution for several months [White *et al.*, 2006], ultimately resulting in the investment in a new treatment plant. Thus, our predictions, showing that the likely interruptions last months to a year, are consistent with some of the observed impacts from large erosion events in the region. The observations also provide some justification for the approach whereby the model considers only the large, threshold-driven erosion response in determining the risk to water quality.

We resort to some simplification and assumptions related to the flow delivering sediment to the reservoir boundary and the propagation of sediment within the reservoir itself. Source of error in model prediction can be evaluated separately for the two model components (Table 3). For sediment delivery by debris flows, the data used in the model development can be used to quantify the magnitude of error caused by debris flow thresholds, the predicted sediment loads and the clay content. For the propagation of sediment within the reservoir, the error was quantified with respect to water temperature, seasonality, the transfer function (Eqn. 1), which comprise errors that stem from variable input locations and assumed suspended sediment threshold for treatment. The magnitude of the error terms depends on the magnitude of  $D_{>th}$ . For  $D_{>th} = 30$  days (1 month), the transfer function (Eqn. 1) is a major source of error, with RMSE (52 days) that is very high ( $\pm 177\%$ ) relative to the predicted duration of treatability disruption. The relative importance of this error term decreases as the magnitude of impact increases; however, it remains the largest source of error in the model. Developing a narrower threshold range for suspended sediment and treatability would greatly



associated with the calibration of radar reflectivity with measured rainfall. Across the network of rain gauges ( $n=48$ ) used in calibrating the Melbourne Rainfall Radar the mean RMSE for rainfall intensities  $> 5 \text{ mm h}^{-1}$  is  $5.2 \text{ mm h}^{-1}$ . This is low relative to the intensities that trigger debris flows ( $I_{12} > 50 \text{ mm h}^{-1}$ ) and unlikely to have large impact on the results. Second, in our implementation of the rainfall generator model, we assume for simplicity the coefficient of variation (CV) in rainfall intensity and wat area ratio (WAR) are kept constant (parameters described in Supporting Information). In addition, the rainfall spatial correlation is assumed to be constant; the duration of the available radar data (5 years) was insufficient for extracting parameters that vary with storm intensity.

### *5.2 Costs and opportunities for mitigation*

The potential costs of wildfire related contamination events in forested water supply catchments are substantial. There are a number of factors to consider when assessing impacts on water supply and costs. Management of water offtakes by regulating the depth of water extraction is one strategy that is likely to provide a means for reducing the duration that the UY reservoir is offline. Our risk analyses points to the large importance of expanding on the reservoir hydrodynamics modelling to better understand the capacity to mitigate risks with reservoir operations. The availability of alternate offtakes at different depths and understanding the location and seasonal behavior of the thermocline are important in risk mitigation. Expanding on our hydrodynamic modelling and developing a framework for evaluating how reservoir operations, hydrodynamics and boundary conditions impact on risk should be a priority as it would greatly improve preparedness to respond in the event of wildfire, and improve the knowledge base needed to make strategic decisions about investment in infrastructure that can reduce the overall vulnerability of the water supply systems to threats from post-fire erosion.

Another key point to note regarding risk mitigation is that the source of the threat is relatively concentrated in a small area of the UY catchment (Figure 6). Therefore, efforts to mitigate risk can be highly targeted. Efforts to reduce sediment loads from debris flow may include channel structures to prevent sediment from entering the reservoir [e.g. check dams: *Abbasi et al.*, 2019] or hillslope treatments that trap hillslope sediment, modify the hydrology, and reduce the probability of a debris flow initiating [*Robichaud et al.*, 2013]. The initiation of debris flows in post-fire settings is highly sensitive to peak runoff [*Kean et al.*, 2011], so any successful attempts to reduce runoff has the potential to substantially reduce the likelihood of a debris flow. There is potential to reduce the probability of large costs if high severity can be limited [*Jones et al.*, 2017]. However, wildfire cannot be eliminated from these ecosystems, and studies indicate that there is not much leverage to reduce the annual area burned by wildfire with fuel reduction methods such as planned fire [*Price et al.*, 2015]. Nonetheless, our analysis indicates that by modifying fire severity, large reduction in risk can be achieved.

## **Conclusion**

Water supply reservoirs in forested catchments are vulnerable to post-fire erosion and sediment transport. However, the risk from such impacts has until now been difficult to quantify because of the lack of suitable models and model integration to represent and link the dominant processes, from rainfall properties, to catchment vulnerability and reservoir propagation of pollutants. In addition, little attention has been given to the development of pragmatic metrics of risk that are relevant and informative for water supply managers and policy makers and enable the cost/benefit of mitigation options to be evaluated easily and transparently. This research has addressed these limitations by coupling reservoir

hydrodynamics, post-fire erosion potential, and stochastic rainfall to estimate the probability and duration that turbidity thresholds for treatability are exceeded at the reservoir offtake.

The modelling approach is parsimonious and can be parameterized spatially, focusing the process representation on the dominant controls on the output metric (for example, by representing only debris flow processes). Numerical constraints associated with three dimensional hydrodynamic modelling of a vast number of sediment input realizations (due to the stochastic nature of wildfire and rainfall patterns) have been overcome successfully using a parameter reduction approach to generate a simple relationship between the output metric (probability of days undeliverable) and the input sediment loads from the debris flow process. Lastly, we apply our model to the Upper Yarra catchment that directly delivers water to ~ 4 million residents of Melbourne in SE Australia and show that after high severity wildfire there is a relatively high probability ( $0.1 < AEP < 0.3$ ) of major consequences in the UY reservoir whereby water is untreatable for several months to a year. The duration of this impact is consistent with recent observations of debris flows in other water supply reservoirs in SE Australia.

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## Figures

Figure 1. a) Model framework showing the workflow for producing a risk metric from transfer functions that couple an erosion model with a reservoir hydrodynamics model. b) Example of the final risk metrics which is the probability of consequence.

Figure 2. Map showing regional context and the Upper Yarra (UY) catchment. a) A digital terrain model (20m) of eastern upland of Victoria with i) location of post-fire debris flows used in developing a model of debris flow magnitude, ii) the location of experiment sites used to evaluate the surface runoff model and, iii) wildfires in 2007 and 2009 that were used in calibration and testing the erosion model (see Supporting Information). Wildfires from 2009 (difference Normalized Burn Ratio, or dNBR, shown in map) were used in developing wildfire scenarios in the UY catchment, where the model is implemented. The dNBR was calculated from Landsat imagery using methods described in *Key and Benson* [2005] and scaled by a factor of  $10^3$ . b) The distribution of forest types in the UY catchment and the 4 locations (Site 1-4) where sediment input was simulated in the hydrodynamic model.

Figure 3. Fire severity distributions for zero-order headwaters in the Kilmore-Murrundindi (KM) fire. High and low fire severity distributions are obtained for areas that burned before and after 10pm on the day that the fire started (see Figure 2a). High and low severities generally correspond to crown and understory fires, respectively.

Figure 4. The concentration of total suspended clay at the dam wall offtake for low and high clay inputs ( $M_c$ ) at sites 1-4. a, b) Site 1, c-d) Site 2, d-e) Site 3 and e-f) Site 4. The thick line illustrates total suspended clay (grain size  $\leq 0.002$  mm), with each gray line indicating contribution from size fractions 0.001 - 0.002 mm. Dashed line horizontal line is the upper limit ( $10 \text{ mg L}^{-1}$ ) of the treatability threshold.

Figure 5. Relations between the number of days that suspended sediment concentrations exceed water quality thresholds at the water offtake given different sediment loads input at sites 1-4 for suspended sediment concentration thresholds of  $5 - 10 \text{ mg L}^{-1}$  (which correspond to  $\sim 5$  NTU).

Figure 6. Distributions of debris flow rainfall thresholds ( $I_{12}$ ) and annual exceedance probabilities (AEP) in headwaters for high fire severity (a, b) and low fire severity (c, d). Values are the median of 10 samples of fire severity from the distributions in Figure 3.

Figure 7. Scenarios of clay delivery ( $M_c$ ) to the Upper Yarra reservoir and disruptions to treatability as a result of post-fire debris flows for high severity wildfire (a,b) and low severity wildfire (c,d). Annual exceedance probabilities (AEP) for  $M_c$  are obtained using design storms with depth area reduction factor and rain fields from Space-Time Realizations for Areal Precipitation (STREAP). For both rainfall types, the outcome from storms with the same AEP are variable depending on spatial distribution of peak intensities. All distributions are equally likely. The conversion from  $M_c$  to days of untreatable water stems from equation 1 where  $D_{>th} = 0.51M_c^{0.77} - 44$  (Figure 5), and assumes a treatability threshold between  $5$  and  $10 \text{ mg L}^{-1}$  ( $\sim 5$  NTU). Results in the shaded region of c) and d) fall outside the range of sediment input scenarios used in the hydrodynamic simulations (Figure 5).

## Tables

Table 1. Change in the simulated number of days the water is undeliverable water at Site 1 between events in summer and autumn.

| Site 1  | Limit value | Summer | Autumn | Change |
|---------|-------------|--------|--------|--------|
| Lowest  | 5 mg/l      | 7 d    | 0 d    | -      |
| Highest | 5 mg/l      | 231 d  | 303 d  | +31%   |
|         | 7 mg/l      | 170 d  | 257 d  | +51%   |
|         | 10 mg/l     | 107 d  | 152 d  | +42%   |

Table 2. Annual exceedance probabilities (AEP) for durations of water supply interruptions ( $D_{>th}$ ) for simulations using two different methods of generating rainstorms.

| Days exceeding threshold ( $D_{>th}$ ) days | Mass of clay delivery [Mg] | AEP design storm [-] | AEP STREAP [-] |
|---|----------------------------|----------------------|----------------|
| <i>Low severity fire</i>                    |                            |                      |                |
| 50  | 876                        | 0.19                 | 0.15           |
| 100   | 1525                       | 0.15                 | 0.09           |
| 300   | 4721                       | 0.06                 | <0.01          |
| <i>High severity fire</i>                   |                            |                      |                |
| 50  | 876                        | 0.45                 | 0.39           |
| 100   | 1525                       | 0.42                 | 0.28           |
| 300   | 4721                       | 0.29                 | 0.10           |

Table 3. The contribution of different model components to error in predictions.

| Components                                  | Method             | StdDev | CV [%] | Uncertainty in $D_{>th}$ [days] <sup>4</sup> |                         |                          |
|---|--------------------|--------|--------|--|-------------------------|--------------------------|
|   |                    |        |        | $D_{>th} =$<br>1 month                       | $D_{>th} =$<br>6 months | $D_{>th} =$<br>12 months |
| DF threshold<br>( $28.3 \leq b \leq 32.2$ ) | Range <sup>1</sup> | 1.13   | 3.7    | ±3.7<br>(12)                                 | ±11<br>(6.2)            | ±20<br>(5.6)             |
| Load [Mg]<br>(obs vs pred)                  | RMSE <sup>2</sup>  | 112    | 13     | ±3.5<br>(12)                                 | ±20<br>(11)             | ±41<br>(11)              |
| Clay content [%]<br>(sample distribution)   | From sample        | 0.025  | 3.1    | ±0.8<br>(2.7)                                | ±4.9<br>(2.7)           | ±9.9<br>(2.7)            |
| Input location and turbidity<br>threshold   | RMSE <sup>3</sup>  | 52     | 29     | ±52<br>(173)                                 | ±52<br>(28)             | ±52<br>(14)              |
| Season<br>(autumn or summer)                | Range <sup>1</sup> | 19.1   | 11     | ±3.3<br>(11)                                 | ±20<br>(11)             | ±41<br>(11)              |

<sup>1</sup>Assuming uniform distributions where  $stdev=(max-min)/sqrt(12)$  (see Supporting

Information for description of parameter  $b$ ).

<sup>2</sup>From data in Nyman et al 2013 (see Supporting Information).

<sup>3</sup>From fitted function (Eqn. 1) in Figure 5

<sup>4</sup>Values in () are the error in units of %













